

Bridge Planning Culvert Case Study

BF 71631 Woodpecker Creek

Bridge Planning
Practitioners Workshop
April 2012

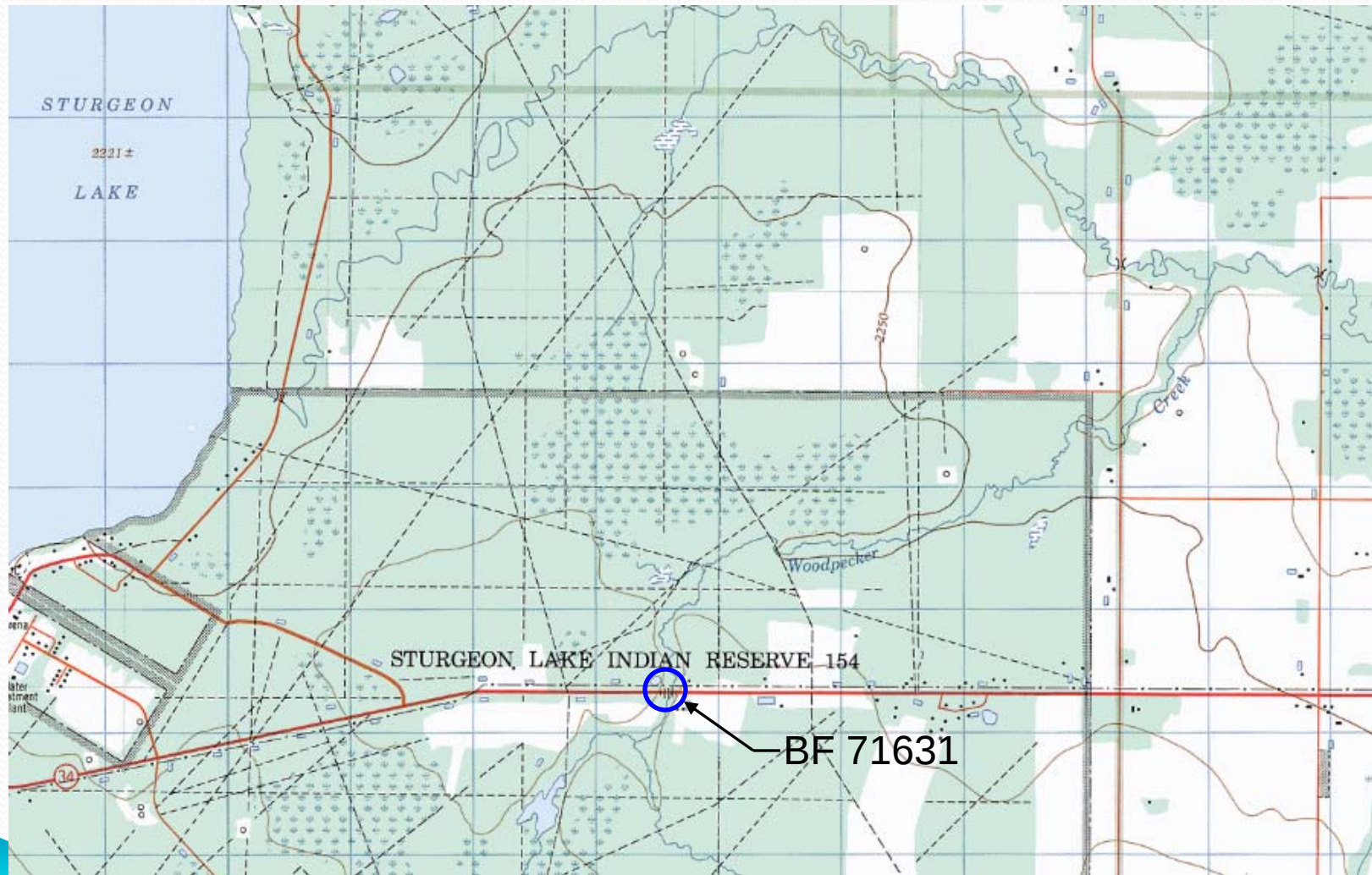


Site Summary

- Located on Highway 43:06
- Tributary to Sturgeon Creek which flows into Sturgeon Lake
- Sturgeon Lake Indian Reserve u/s and d/s
- Information available in HIS
 - Drainage area = 54 km²
 - $S = 0.0033$
 - $B = 3 \text{ m}$, $h = 1.5 \text{ m}$, $T = 6 \text{ m}$
 - $Y = 2.3 \text{ m}$, $V = 1.6 \text{ m}$, $Q = 18 \text{ cms}$



Site Summary



File History

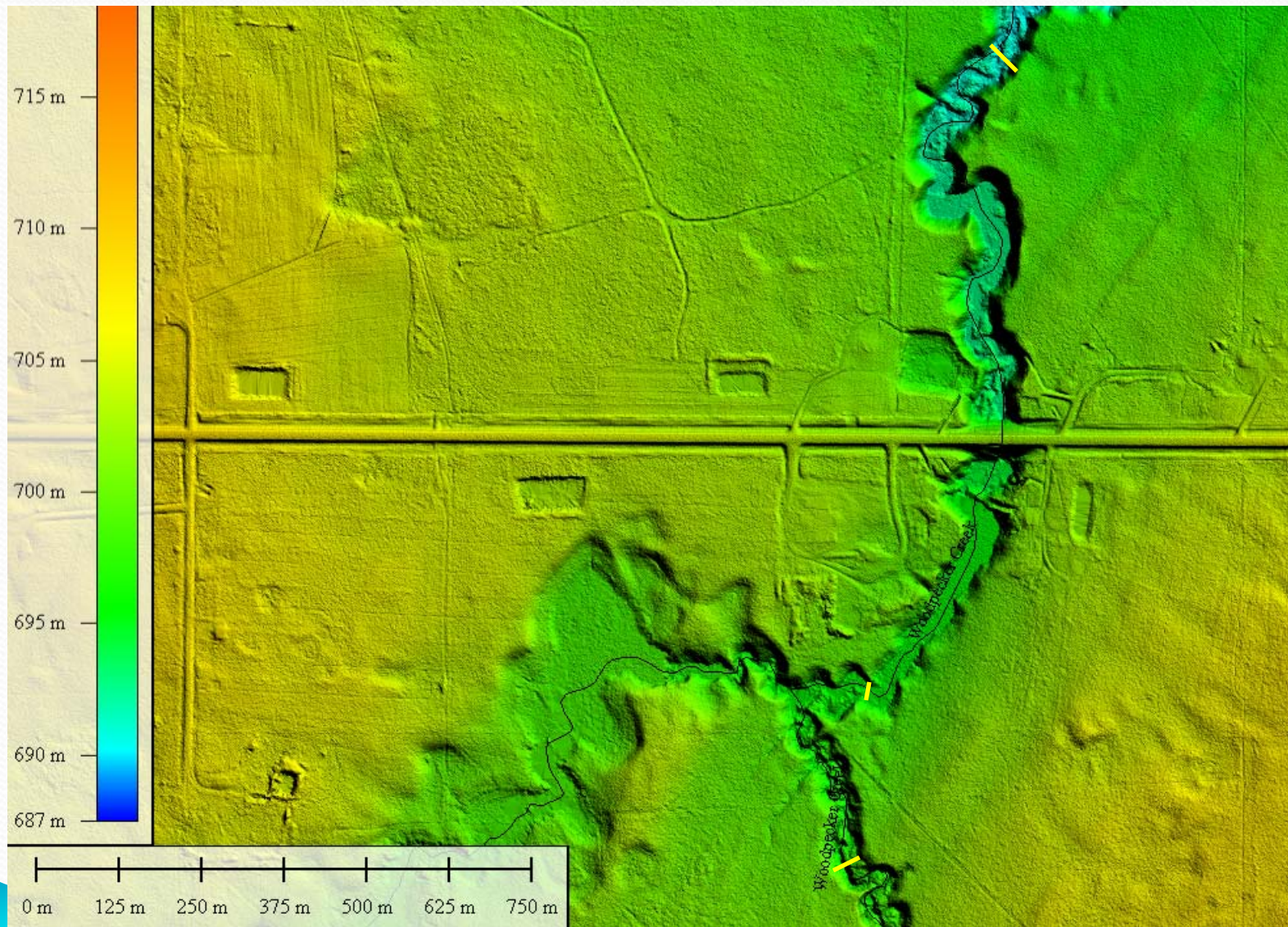
- 1931 original bridge built, 3-18' spans, dh=20'
- Bridge replaced in 1944, 4-18' spans
- 1954 – hydrotechnical summary
 - $B = 10'$, $T = 18'$, $h = 4'$, $Y = 5-8'$, $Q = 200$ cfs
- 1954 installed 96" CSP x 224'
- 1963 HW (Apr) : $0.6\text{m} > u/s$ crown
- 1966 HW (Apr) : $Y \sim 0.5 - 1.0\text{ m} < u/s$ crown,
 $Y \sim 1\text{ m d/s}$
- 1979 Erosion at both ends repaired with rock
- 1988 BIM HWM $Y = 1.6\text{ m}$

File History

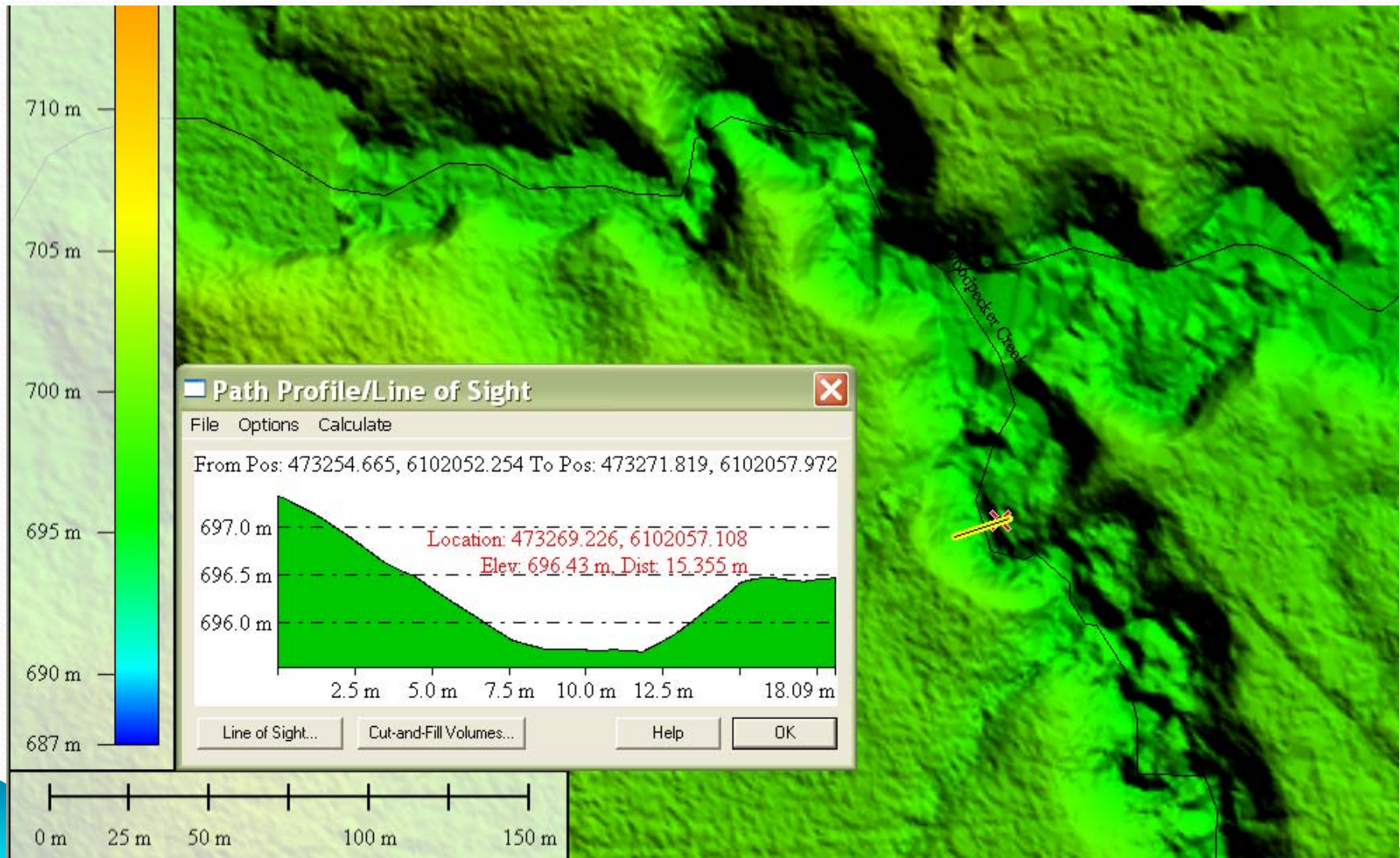
- 1992 Photo: $B = 3 \text{ m}$, $h = 1.5 \text{ m}$, $T = 6 \text{ m}$, lots of drift.
- 1993 Report: $Q = 27 \text{ cms}$, $Y = 2.1 \text{ m}$, $V = 1.5 \text{ m/s}$
- 1996 Photo: drift to $\sim 0.5 - 1.0 \text{ m}$ below u/s crown
 - Note : $DA = 54 \text{ km}^2$, $Q = 18 \text{ cms}$, $S = 0.005$, rec. 4.3m SP
- 1999 Design
 - $DA = 56 \text{ km}^2$, $Q = 23 \text{ cms}$, $S = 0.005$, $Y = 2.0 \text{ m}$, recommended 4.3 m CSP x 110 m
 - Photo: $B = 3 \text{ m}$, $h = 1.5 \text{ m}$, $T = 6 \text{ m}$, ponding u/s behind beaver dam at inlet, scour hole d/s



DEM



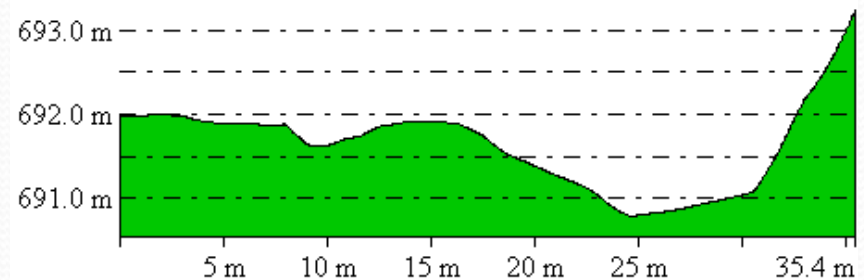
DEM – cutting stream sections



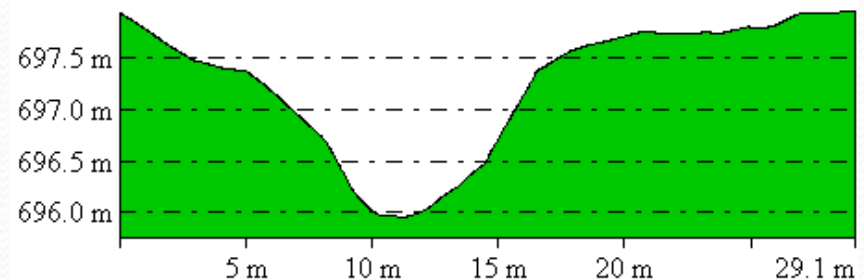
Estimating Channel Parameters

- Note differences between sections
- Use judgment and estimate B, h and T
- Visit the site to confirm estimates

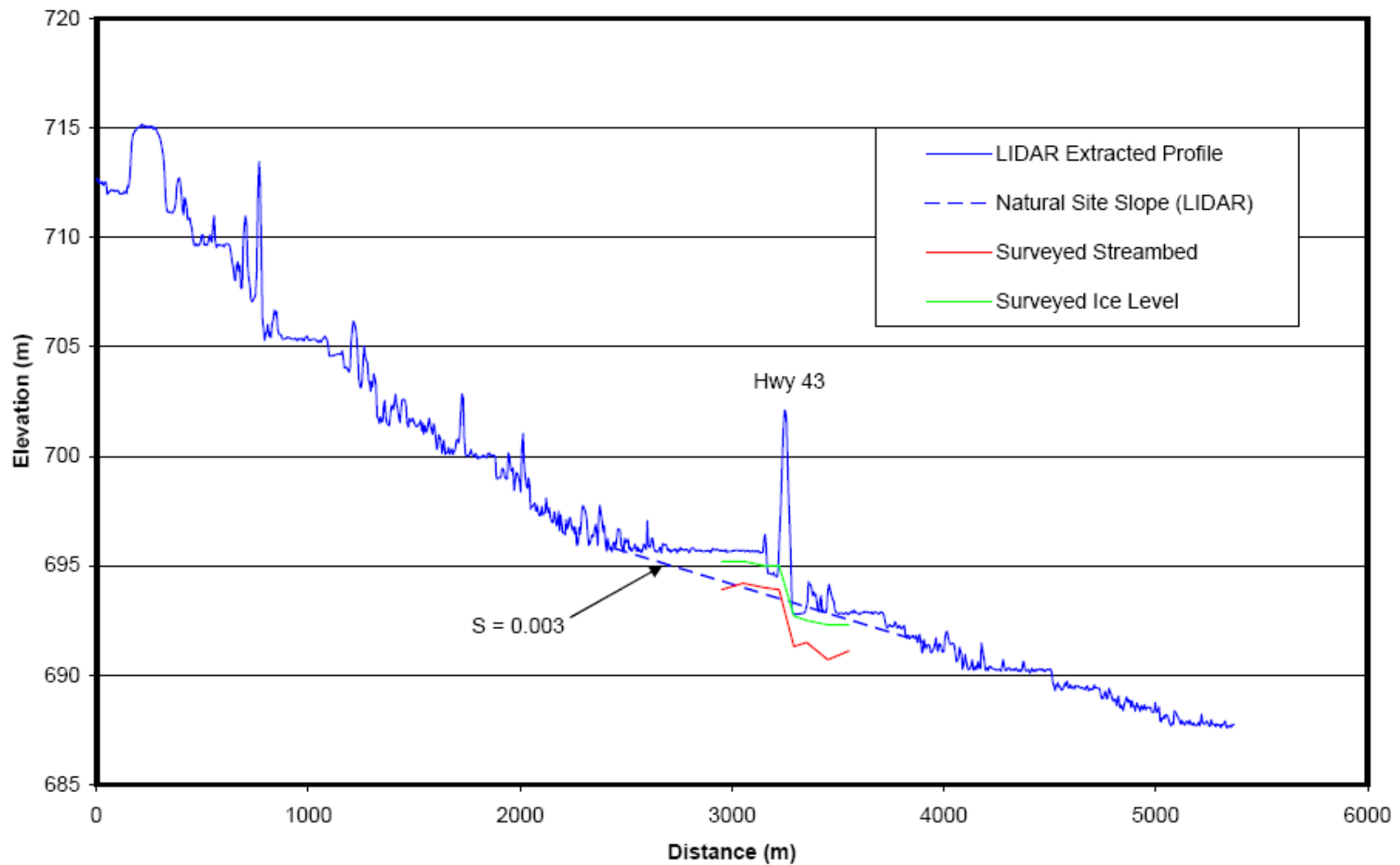
From Pos: 473545.464, 6103079.950 To Pos: 473564.763, 6103050.260



From Pos: 473317.089, 6101872.030 To Pos: 473344.933, 6101880.597



Estimating Slope



- Discuss the B, T and h values presented. Do you agree with them why or why not? Do you think it will impact the magnitude of the design parameters?
- How would you propose to deal slope issue at this site? Why is there a difference between the LiDAR and survey?
- What are some of the bridge planning issues at this site? Remember to broaden your view away from just the crossing.



Estimating Slope

- Slope variation due to aggradation u/s, scour downstream
- set the invert at a slope of 0.0033 and match the d/s channel with burial
- Create a transition to the upstream channel that will self correct over time



Hydrotechnical Summary

- Channel Capacity
 - $S = 0.0033$
 - $B = 3 \text{ m}$, $h = 1.5 \text{ m}$, $T = 6 \text{ m}$
 - Use $n = 0.05$
 - At Bank Height : $Y = 1.5 \text{ m}$, $V = 1.1 \text{ m/s}$, $Q = 7 \text{ cms}$
 - At Channel Capacity : $Y = 2.3 \text{ m}$, $V = 1.6 \text{ m/s}$, $Q = 18 \text{ cms}$



Hydrotechnical Summary

- Historical Highwater Data
 - April 1963 (0.6 m > u/s crown, Q ~ 15 cms)
 - April 1966 (~0.8m < u/s crown)
 - 1988 BIM HWM was noted with Y = 1.6 m
 - 1996 photo shows drift to about 0.8 m < u/s crown
 - Past designs have been based on Q = 17 – 27 cms
 - No WSC gauges on this stream
 - Gauges in the area suggest HW in July 1971 and June 1996.
 - Largest storms in the storm database for this area are about 130 mm in July 1987 and about 100 mm in Jun 1961, August 1976, July 1982, and June 1993

Hydrotechnical Summary

- Basin Runoff Potential
 - $d = 70 \text{ mm}$, $T_p = 20 \text{ hrs}$
 - Recall that $q \text{ (m}^3\text{/s)} = d/(3.6 * T_p)$, therefore $q = 1.0 \text{ cms/km}^2$
 - Gross DA = 54 km^2
 - $Q_p = q * \text{DA}$, therefore $Q_p = 54 \text{ cms}$

Hydrotechnical Summary

- Design Discharge Recommendation
 - The channel capacity estimate is consistent with historic observations and is within the basin runoff potential
 - Recommended parameters : $Y = 2.3 \text{ m}$, $V = 1.6 \text{ m/s}$,
 $Q = 18 \text{ cms}$.



Consultant Hydrotechnical Recommendation

- Completed more on site investigation
- Recommend $Y = 2.2$ m, $V = 1.6$ m/s, $Q = 25$ cms

Channel Capacity Calculator

				h	Y_{cc}	Y_{spec}
S	0.00330	Y		1.5	2.25	2.2
B	3.0	A		9	16	15.3
h	1.5	d		0.9	1.7	1.6
T_h	9	V		1.1	1.6	1.6
Roughness	0.050	Q		9.8	25.5	24.3
HDG 'n'	0.050					

Culvert Sizing

- Investigated several alternatives including:
 - Several sizes of SPCSP
 - 4.5 m x 7 m HESPCSP
 - 7 m x 14 m steel or concrete arch
- Remember the design process is iterative



Hydraulic Analysis - Input

- Flow Profile tool
- Template screen and Input Screen
- Note different slopes on Input screen

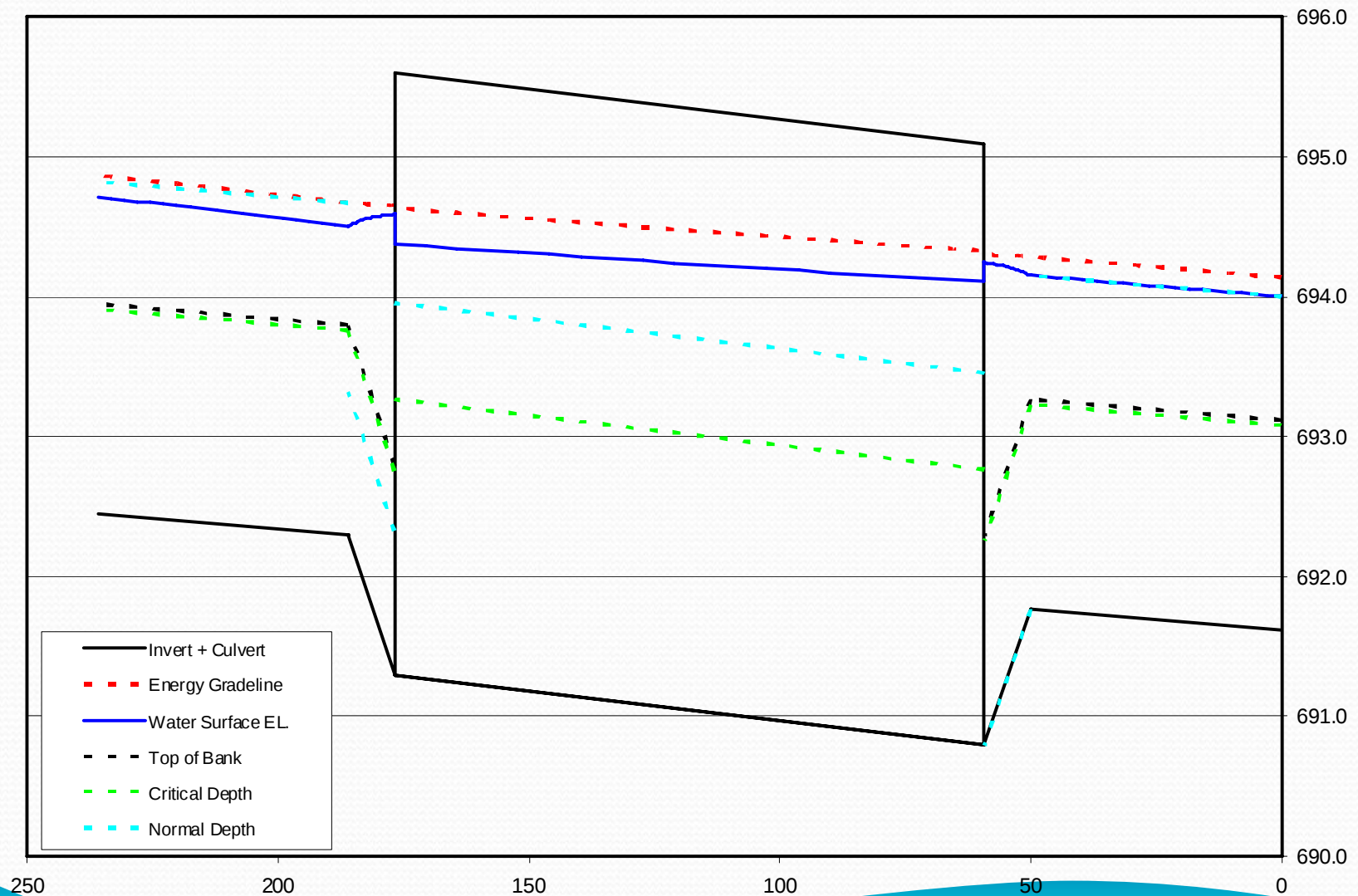
Typical Channel		Single Culvert		Bridge	
B	3	Shape	R	B	8
h	1.5	Rise	4.3	m	2
T	8	Span		W	20
S	0.0030	Length	117.7		
Elev	691.8	Burial	1		
		L _{trans}	9.2		
			Prep Culvert Data		Prep Bridge Data

Boundary Conditions												
Q (cms)		25										
D/S BC Type		Run										
No.	Section Description	U/S Elev	D/S Elev	Length	Shape	Rise / h	Span / B	T	n	Substrate	n used	S
1	D/S Channel	691.8	691.6	50	T	1.5	3	8			0.05	0.003
2	D/S Transition	690.8	691.8	9.2	T	1.5	3	8	0.05		0.05	-0.1057
3	Culvert	691.3	690.8	117.7	R	4.3					0.028	0.00426
4	U/S Transition	692.3	691.3	9.2	T	1.5	3	8	0.05		0.05	0.1087
5	U/S Channel	692.5	692.3	50	T	1.5	3	8			0.05	0.00306

- Summary of output from Flow Profile tool, Results tab

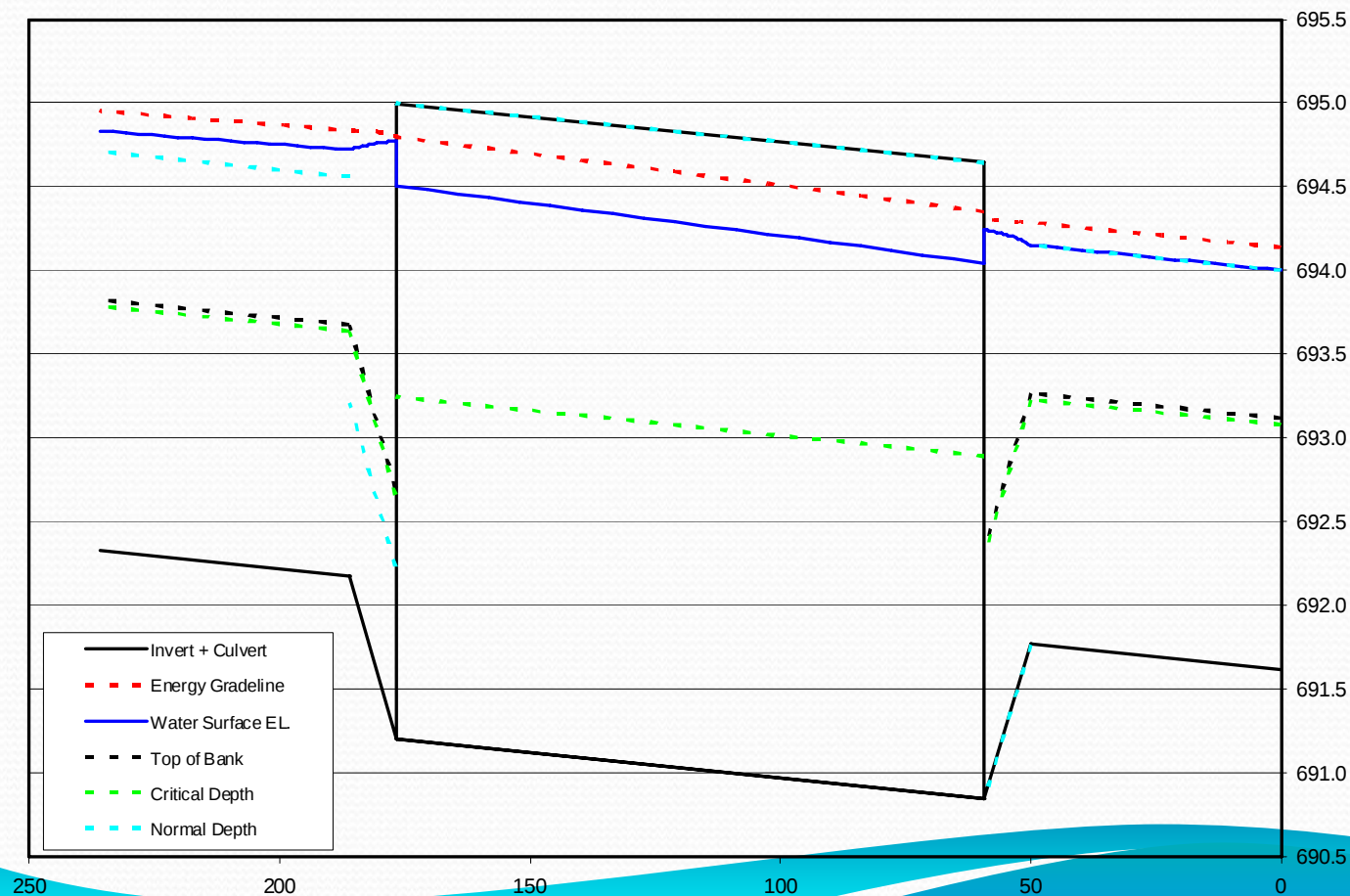
[illegible]

Hydraulic Analysis - Output



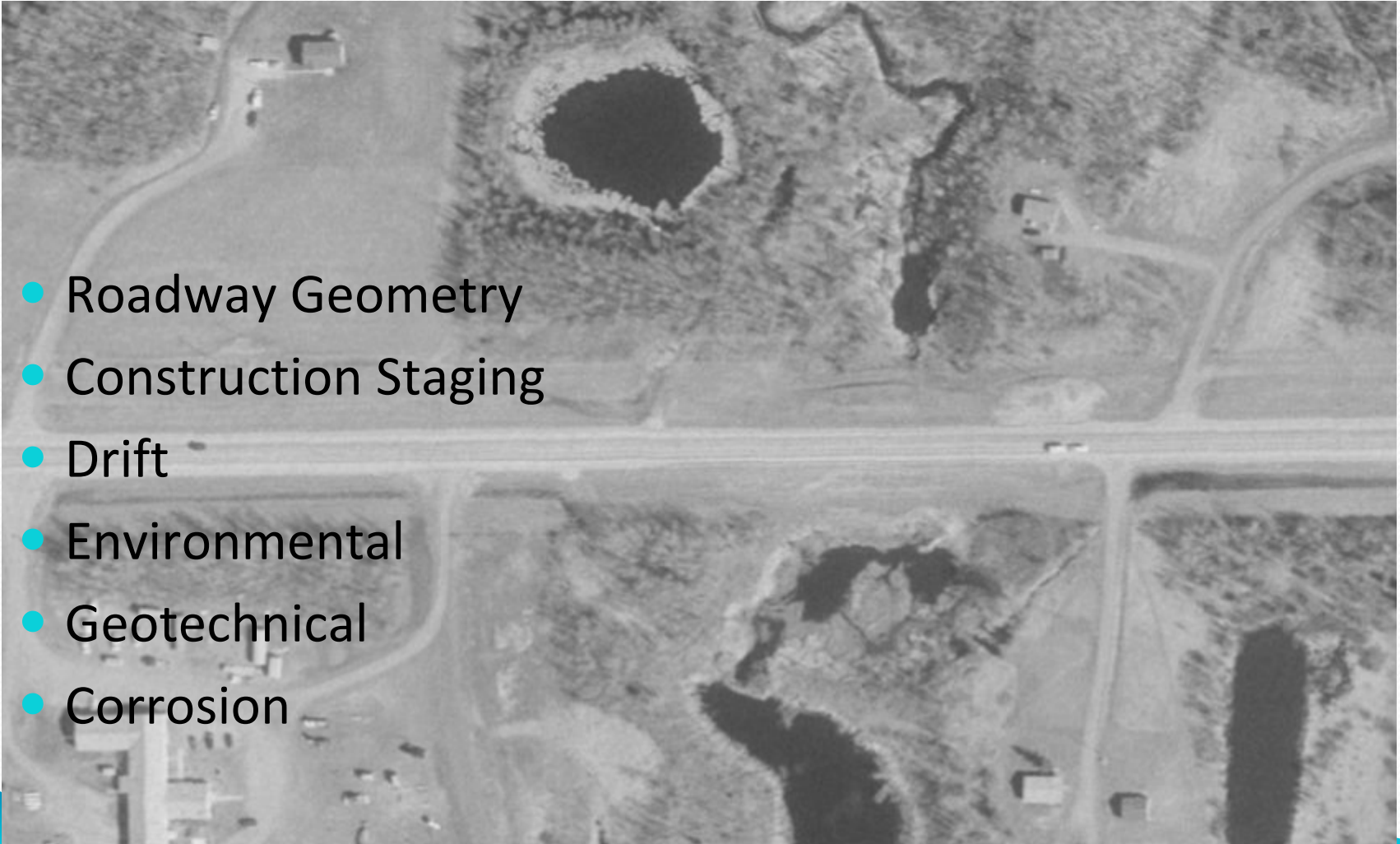
Hydraulic Analysis – Alternate Size

- Modify culvert diameter to 3.8 and change embedment to 0.95 m



Other Design Considerations

- Roadway Geometry
- Construction Staging
- Drift
- Environmental
- Geotechnical
- Corrosion

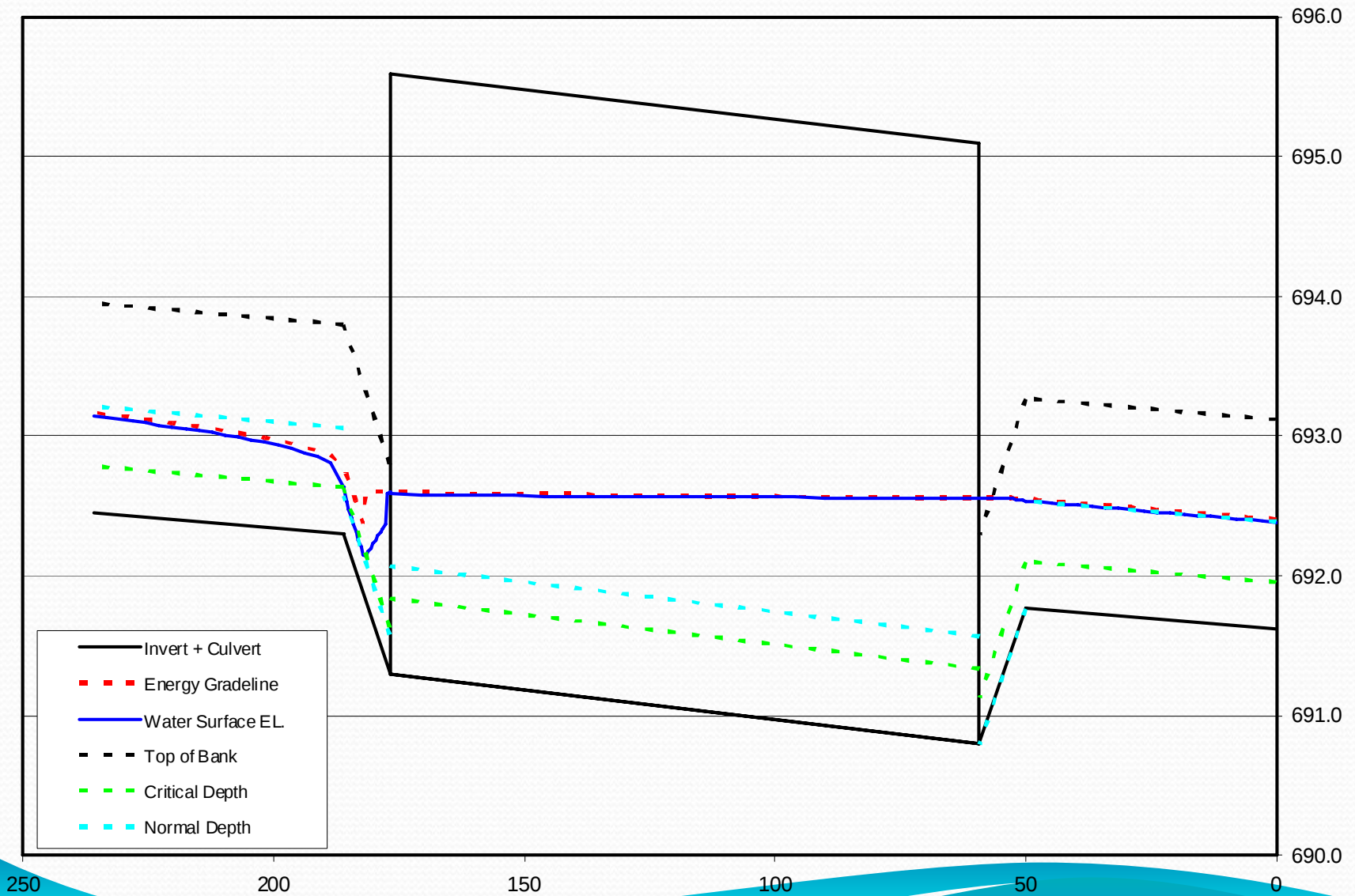


Fish Passage

- Low potential for large bodied fish utilization
- Due to potential check passage
- Fish flow Q = 2 cms, increased roughness to 0.06 (decreased depth)

	Boundary Conditions												
	Q (cms)	2			Run								
	D/S BC Type												
No.	Section Description	U/S Elev	D/S Elev	Length	Shape	Rise / h	Span / B	T	n	Substrate	n used	S	
1	D/S Channel	691.8	691.6	50	T	1.5	3	8	0.06		0.06	0.003	
2	D/S Transition	690.8	691.8	9.2	T	1.5	3	8	0.06		0.06	-0.1057	
3	Culvert	691.3	690.8	117.7	R	4.3			0.035		0.035	0.00426	
4	U/S Transition	692.3	691.3	9.2	T	1.5	3	8	0.06		0.06	0.1087	
5	U/S Channel	692.5	692.3	50	T	1.5	3	8	0.06		0.06	0.00306	

Fish Passage



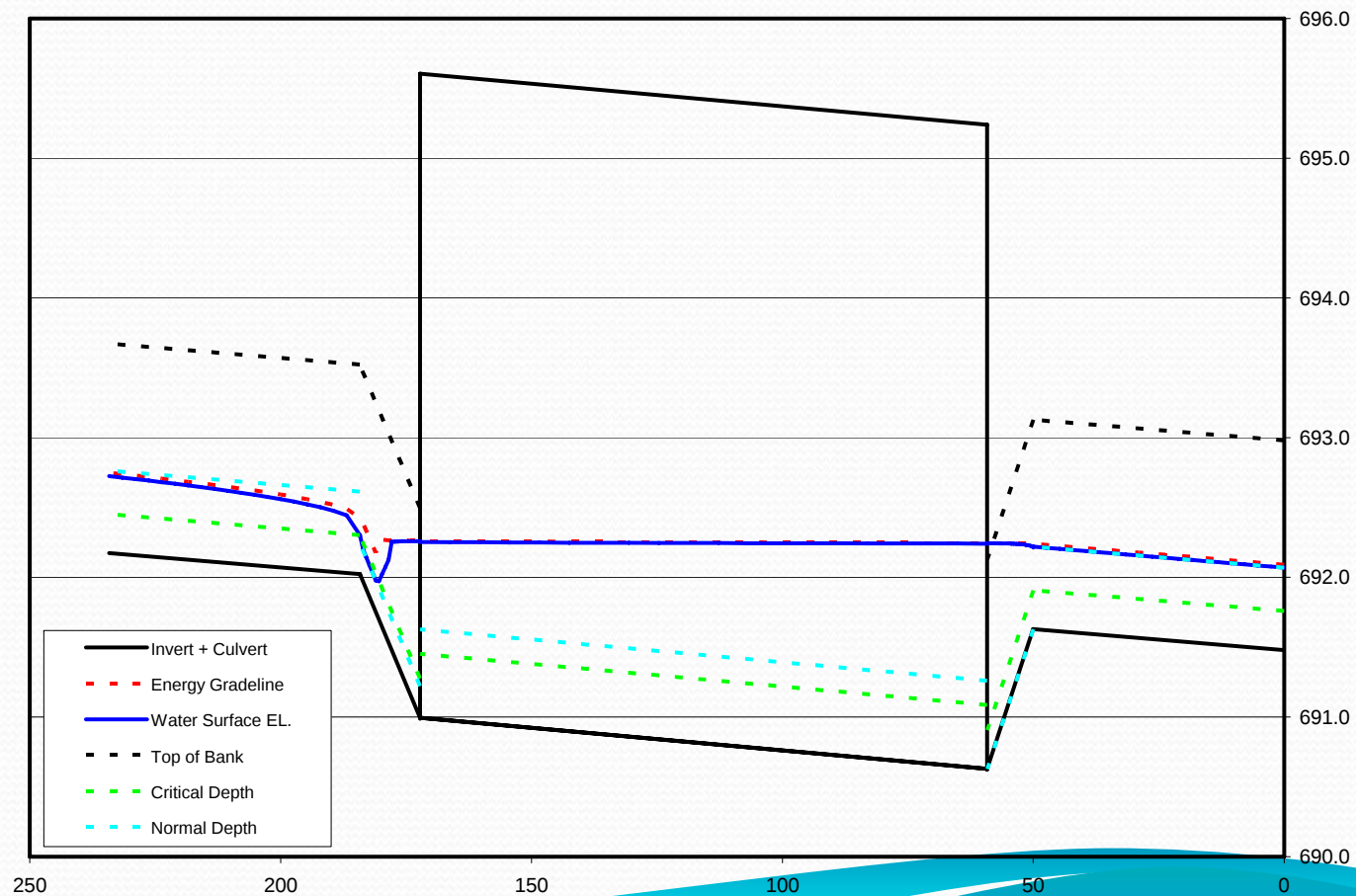
Fish Passage

Element	Station	X	Y	V	Invert	Crown	WL	EGL	
	(m)	(m)	(m)	(m/s)	(m)	(m)	(m)	(m)	
4	176.85	0.00	1.30	0.30	691.297	692.797	692.595	692.599	
4	177.33	0.48	1.24	0.32	691.350	692.850	692.594	692.599	
4	177.82	0.97	0.97	0.45	691.402	692.902	692.368	692.600	
4	178.30	1.45	0.88	0.51	691.455	692.955	692.339	692.600	
4	178.79	1.94	0.80	0.57	691.508	693.008	692.311	692.600	
4	179.27	2.42	0.72	0.66	691.560	693.060	692.283	692.601	
4	179.76	2.91	0.64	0.76	691.613	693.113	692.255	692.601	
4	180.24	3.39	0.56	0.90	691.665	693.165	692.227	692.602	
4	180.72	3.87	0.48	1.09	691.718	693.218	692.199	692.603	
4	181.21	4.36	0.40	1.36	691.771	693.271	692.171	692.603	
4	181.69	4.84	0.32	1.77	691.823	693.323	692.143	692.605	
4	182.18	5.33	0.28	2.09	691.876	693.376	692.153	692.375	
4	182.66	5.81	0.28	2.09	691.929	693.429	692.205	692.428	
4	183.14	6.29	0.28	2.09	691.981	693.481	692.258	692.480	
4	183.63	6.78	0.28	2.09	692.034	693.534	692.311	692.533	
4	184.11	7.26	0.28	2.09	692.086	693.586	692.363	692.586	
4	184.60	7.75	0.28	2.09	692.139	693.639	692.416	692.638	
4	185.08	8.23	0.28	2.07	692.192	693.692	692.471	692.689	
4	185.57	8.72	0.28	2.02	692.244	693.744	692.529	692.738	
4	186.05	9.20	0.33	1.68	692.297	693.797	692.631	692.776	
5	186.05	0.00	0.34	1.67	692.297	693.797	692.633	692.776	
5	188.68	2.63	0.51	1.03	692.305	693.805	692.812	692.866	
5	191.31	5.26	0.54	0.96	692.313	693.813	692.850	692.897	
5	193.94	7.89	0.56	0.91	692.321	693.821	692.882	692.924	
5	196.58	10.53	0.58	0.87	692.329	693.829	692.908	692.947	
5	199.21	13.16	0.59	0.84	692.337	693.837	692.932	692.968	
5	201.84	15.79	0.61	0.82	692.345	693.845	692.952	692.987	
5	204.47	18.42	0.62	0.80	692.353	693.853	692.972	693.004	
5	207.10	21.05	0.63	0.79	692.361	693.861	692.989	693.021	
5	209.73	23.68	0.64	0.77	692.370	693.870	693.006	693.037	
5	212.37	26.32	0.64	0.76	692.378	693.878	693.022	693.052	
5	215.00	28.95	0.65	0.75	692.386	693.886	693.027	693.066	

- Target velocity is 0.61 m/s

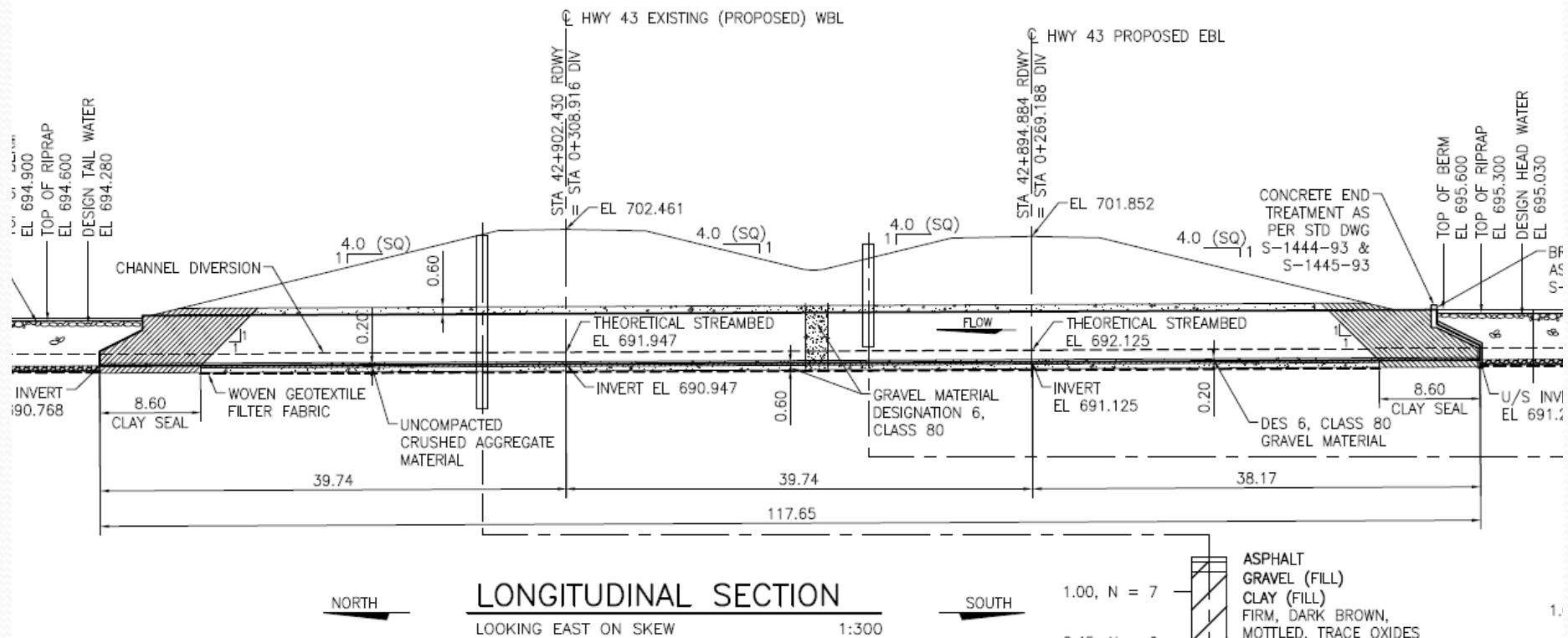
Fish Passage

- Things to try to reduce velocity - increase roughness, flatten slope

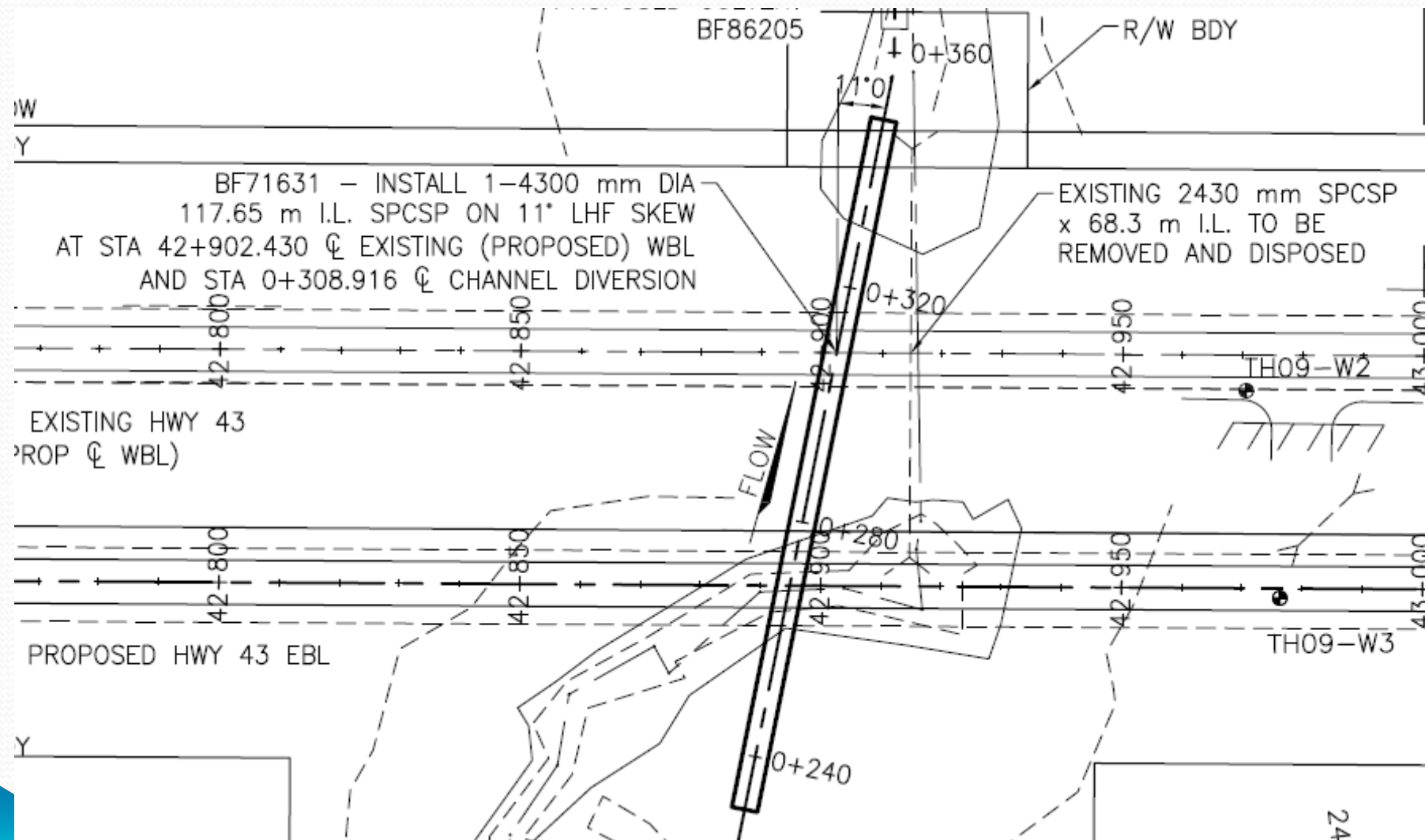


Recommended Design

- 4.3 m x 117.65 m SPCSP



Recommended Design



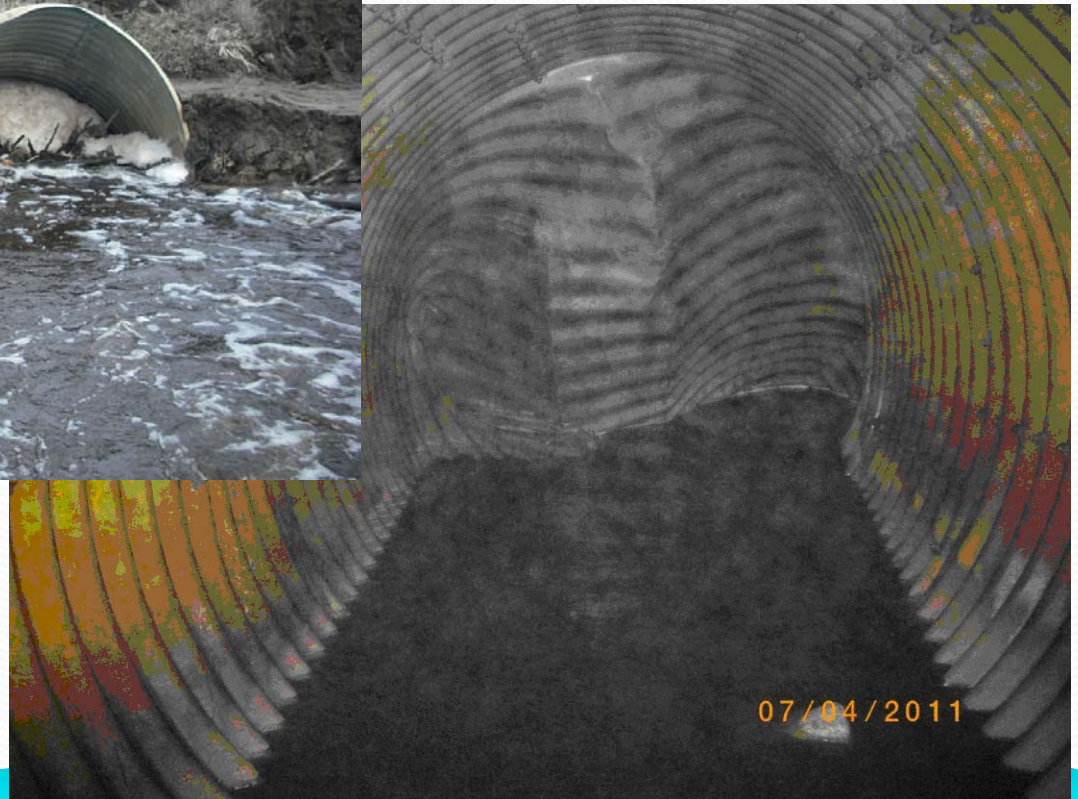
And then what happened?



- July 2011 flood
- Culvert failed – uplift at inlet



Uplift failure



Installing liner



Installed liner (temporary)

